



ABOUT ENGINEERING AND GEOLOGICAL SURVEYS OF SOIL PROPERTIES IN THE CONSTRUCTION OF APARTMENT BUILDINGS

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Abstract: Engineering and geological surveys are summative concepts, the essence of which is to conduct a study of the geological properties of the soil on the territory of the construction site. The main goal is to obtain all the necessary materials to fully justify the possibility of designing and building any object on the selected territory. This study describes

engineering and geological surveys of soil properties for the preparation of project documentation for the construction of an apartment building at the address Kirov region, Vyatskopolyansky district, village. Krasnaya Polyana, Druzhba str., 1. Statistical processing of physical characteristics of the studied soils, as

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well as indicators of physical and mechanical properties of soils are presented.

Keywords: laboratory engineering and geological surveys, project documentation, soil properties, physical characteristics, statistical processing, reconnaissance survey, drilling, soil sampling, sounding, field stage, laboratory stage, Desk processing.

Engineering and geological surveys are a group of studies aimed at determining the geological and hydrogeological conditions in the area, intended for the future construction of a private house, apartment building, business object, linear highways, communication networks and any other capital structures [5].

The primary task of engineering and geological surveys is to determine the properties of the soil to select the configuration of the foundation-the basis of the future structure. Depending on the strength of the soil base, its corrosion activity, the depth of freezing, as well as the nature of underground water and its chemical composition, the type and depth of the foundation is determined,

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and the need for additional measures for protection and strength.

Engineering and geological surveys include several stages, the implementation of which is prescribed by the SNiP standards. The work includes [1]:

1. Preparation. Reconnaissance of the area, topographical research, study of archival materials on the site, determining the scope of upcoming work and the choice of methods.

2. The field phase. Drilling operations, sounding and stamp testing of soils, description of the geological and lithological structure of the soil section, sampling of soil samples and groundwater.

3. The laboratory stage. It is carried out in parallel with field work. In a certified chemical-soil laboratory, soil studies are conducted for physical and mechanical parameters: plasticity, fluidity, resistance to precipitation, capillarity, chemical composition

and degree of corrosion aggressiveness are determined.

4. The Desk phase. Processing of the data obtained using computer programs and mathematical calculations, drawing up a description of geological conditions, forecasting possible changes



and the risk of dangerous natural processes, making a Technical report.

The object of this study is the properties of soils in the construction of multi-apartment residential buildings.

The purpose of this study is to conduct laboratory engineering and geological surveys of soil properties for the preparation of project documentation for the construction of an apartment building at the address: Kirov region,

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Vyatskopolyansky district, village. Krasnaya Polyana, Druzhba str., 1.

So, let's imagine the composition and scope of work for engineering and geological surveys of soil properties during the construction of an apartment building at the address Kirov region, Vyatskopolyansky district, village. Krasnaya Polyana, Druzhby str., 1. in the form of table 1.

Table 1. Composition and scope of work

Nº	Name of works	Unit of measurement	Quantity	Note
Field work				
1	Reconnoitering examination	km ²	0,2	Corrected during work
2	Soil sampling	Sample	20	
Laboratory works				
3	Determination of physical properties of soil	Determ.	20	Corrected during work
Cameral works				
4	Cameral treatment of the laboratory data	Sample	20	Corrected during work

2. PROPOSED METHODOLOGY

To accomplish the tasks, a set of semi-laboratory and laboratory tests will be carried out to obtain data recording the state and physical properties of soils, identifying dangerous geological and engineering-geological processes.

Administratively, the site is located in the western part of the urban-type settlement Krasnaya Polyana of the Vyatskopolian district of the Kirov region in a residential building. The city of Vyatskiye Polyany, with the same name as the railway station, is located on the Moscow-Yekaterinburg railway. It is

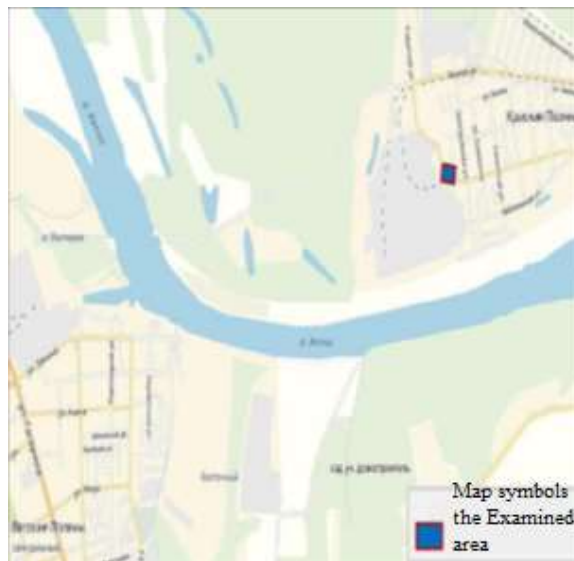


a regional center and it is located 18 km from the village Krasnaya Polyana. The distance from the city to the regional center (Kirov) is: 588 km by railway and 332 km by motorway . Vyatskopoliansky district is located in the southeastern part of the Kirov region

and borders: in the northwest with the Malmyzhsky district, north and east with the Republic of Udmurtia; in the south and west with the Republic of Tatarstan.

The location of the area being examined is shown in the diagram of the area of work (Fig. 1).

Figure 1 - Location of the area being examined



The field work was completed in May 2019. The breakdown and planned - high-altitude snapping of wells were done instrumentally. The catalogue of coordinates and waypoints of

engineering and geological workings for the studied object is presented in table 1.

Coordinate system - MSC-43

Baltic altitude system

Table 1. Catalogue of coordinates and elevations of engineering and geological workings for the object

№ workings	Coordinates		Waypoints m
	X	Y	
Well 1\	320958,45	2286538,29	65,38

тс3.1			
Well 2\ тс3.2	320974,30	2286510,86	65,38

The testing of the physical characteristics of soils was performed in the soil laboratory of VyatTISIz LLC in Kirov.

In order to conduct laboratory test in accordance with the applicable regulatory documents, the following set of engineering and geological works were carried out in the examined area:

- reconnoitering examination;
- drilling of the wells;
- soil sampling;
- hydrogeological examinations;
- static sounding;
- laboratory works;
- cameral processing.

Reconnoitering examination (engineering-geological) of the area was carried out according to SP 47.13330.2012 [10] and SP 11–105–97 [11] (part I, paragraph 5.4) in order to visually assess the nature of the relief, identify surface manifestations of modern geological and engineering geological processes that can adversely affect the stability of the designed structure, the assessment of possible

changes in the geological environment under the influence of the construction and operation of structures [13-17].

Well drilling was carried out by a drilling rig PBU 2-312 core drill with a diameter of 198 mm, using a drilling soil carrier. During the field period, 2 wells were drilled with a depth of 8.0 m.

The selection of monoliths and samples, their packaging and transportation were carried out in accordance with GOST 12071-2014. In the process of drilling, a log of geological documentation of wells was kept.

After the completion of work, all workings were eliminated by backfilling them with excavated soil.

Static sounding was carried out in the circuit of the structure near the drilled wells, in order to clarify the geological and lithological section, according to the requirements of GOST 19912-2012 [6] by the mounted attachment of Geotest OJSC (Yekaterinburg) to the PBU-2-312 drilling rig based on the vehicle KAMAZ according to the pressing load method

according to GOST 19912-2012. Type of probe was II.

The diameter of the probe base is 35.7 mm, the probe base area is 10 cm², the side surface area is 250 cm². The maximum efforts of the probe indentation as a whole are not less than

10 tons, the probe indentation speed is 1.0 m / min, the type of sounding is without stabilization. Data was recorded through 0.1 m with a TEST-K2M type controller with a maximum scale of 250 divisions.

Table 2. Statistical processing of the physical characteristics of soils (IGE-2 Super hard (α 1 III))

Laboratory number, №	Number of working №	Sampling depth		Density g / cm ³			Natural moisture content (W)	factor of porosity (e)	water saturation coefficient (S _r)	Humidity at the border		Plasticity number (I _p)	liquidity index (I _L)
		from	till	of particles (ρ_s)	Of ground (ρ)	Of dry ground (ρ_d)				Fluidity (W _L)	lamination (W _p)		
541	1	1,2	1,4	2,69	1,92	1,66	15,7	0,921	0,68	23,6	17,6	6,0	- 0,32
542	1	1,6	1,8	2,69	1,95	1,64	17,8	0,642	0,75	22,6	17,7	4,9	0,02
543	1	2,0	2,2	2,69	1,96	1,64	19,7	0,643	0,82	24,2	19,5	4,7	0,04
544	1	2,4	2,6	2,69	1,92	1,62	18,7	0,663	0,76	24,1	18,6	5,5	0,02
645	1	2,9	3,1	2,69	1,95	1,62	20,1	0,657	0,82	24,6	19,7	4,9	0,08
652	2	0,8	1,0	2,69	1,94	1,67	16,5	0,615	0,72	22,2	16,7	5,5	- 0,04
653	2	1,4	1,6	2,69	1,94	1,68	15,6	0,613	0,70	21,1	15,6	5,5	0,00
654	2	2,0	2,2	2,69	1,85	1,59	16,5	0,691	0,63	22,9	16,2	6,7	0,02
655	2	2,6	2,8	2,69	1,88	1,61	17,1	0,676	0,68	23,2	17,4	5,6	- 0,05
656	2	3,0	3,2	2,69	1,92	1,60	19,8	0,676	0,79	25,3	20,1	5,2	- 0,06

Number of Definitions	n	10	10	10	10	10	10	10	10	10	10
Min value	X_{\min}	2,69	1,85	1,59	1,56	0,603	0,63	21,1	15,6	4,7	- 0,32
Max. value	X_{\max}	2,69	1,96	1,68	20,1	0,691	0,82	25,3	20,1	6,7	0,08
Normative value	X_n	2,66	1,92	1,63	17,7	0,649	0,73	23,4	17,9	5,5	- 0,03
Dispersion		0,000	0,001	0,001	3,038	0,001		1,542	2,348		
Mean square deviation	S	0,000	0,033	0,029	1,743	0,029		1,242	1,532		
To variation	V	0,00	0,02	0,02	0,10	0,05		0,05	0,09		
The value of the coefficient with a confidence probability of $\alpha = 0.95$	t_α		1,83								
Accuracy indicator of the average value	ρ_0		0,010								
To reliability	Y_g		1,010								
The calculated value of the bearing capacity	X_t		1,90								
The value of the coefficient with a confidence probability of $\alpha=0,85$	t_α		1,10								
Accuracy indicator of the average value	ρ_0		0,006								
To reliability	Y_g		1,006								
The calculated value of the strain	X_t		1,91								

Laboratory tests were performed to determine the nomenclature and physical characteristics of soils in accordance with:

GOST 5180-2015 "Soils. Laboratory methods for determining physical characteristics"[3];

GOST 30416-2012 "Soils. Laboratory tests. General Provisions"[2];

Min value	X_{\min}	2,69	1,94	1,59	17,2	0,546	0,74	20,8	15,1	5,5	0,31
Max. value	X_{\max}	2,69	2,06	1,74	22,7	0,693	0,91	25,6	20,1	6,8	0,64
Normative value	X_n	2,66	1,98	1,64	20,2	0,638	0,85	23,4	17,2	6,2	0,47
Dispersion		0,000	0,001	0,002	4,616	0,002		3,097	2,683		
Mean square deviation	S	0,000	0,037	0,046	2,149	0,045		1,760	1,638		
To variation	V	0,00	0,02	0,03	0,11	0,07		0,06	0,10		
The value of the coefficient with a confidence probability of $\alpha = 0,95$	t_α		1,83								
Accuracy indicator of the average value	ρ_o		0,011								
To reliability	Y_g		1,011								
The calculated value of the bearing capacity	X_t		1,95								
The value of the coefficient with a confidence probability of $\alpha=0,85$	t_α		1,10								
Accuracy indicator of the average value	ρ_o		0,006								
To reliability	Y_g		1,006								
The calculated value of the strain	X_t		1,96								

The standard values of the indicators of mechanical properties according to IGE-2, 3 are taken according to table. B.2.3 adj. B SP 22.13330.2011 [9], the calculations are performed according to the clause 5.3.18, taking into account table 5.11 SP 22.13330.2011.

3. RESULTS

The values of the indicators of physical and mechanical properties of soils are given in table 4 "Summary table of indicators of physical and mechanical properties of soils."

Table 4. Summary table of indicators of physical and mechanical properties of soils

Name of indicator	soil unit 2	soil unit 3
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		sandy clay (α_1 III)	soft sandy loam (α_2 III).
Soil density	ρ_H	1,92	1,98
	ρ_I	1,90	1,95
	ρ_{II}	1,91	1,96
Porosity factor	e	0,649	0,638
Plasticity index, %	I_p	5,5	6,2
Liquidity index	I_L	-0,03	0,47
Specific cohesion (c), kPa	C_H	15	13
	C_I	10	9
	C_{II}	15	13
Angle of internal friction, degrees	φ_H	27	24
	φ_I	23	21
	φ_{II}	27	24
Module of deformation, MPa	E	16	17
Design resistance, kPa	R_0	263	219
K-ratio	k	1,1	1,1

ρ_I is the calculated indicator for calculations of bearing capacity (with confidence probability $\alpha = 0.95$);

ρ_{II} is the calculated indicator for calculations by deformation (at a confidence level of $\alpha = 0.85$);

K - The coefficient adopted when calculating the resistance of the soil of the base, determined by the formula (5.7) SR 22.13330.2011.

4. CONCLUSION

Thus, we conclude on the basis of laboratory tests. The work site is located in the western part of the urban-

type village of Krasnaya Polyana, Vyatskopolian district, Kirov region. Residential and public buildings are located in the immediate vicinity of the examination site. At the time of the examination, the proposed construction site is free from development and is a wasteland. In geomorphological terms, the site of work is confined to the second floodplain terrace of the river Vyatka. The territory of the examination site has a general decrease in relief in a southerly direction towards r. Vyatka with a slope of $0^\circ 30'$, the surface of the site is planned (previously a residential



building was located on the site). The absolute elevations of the workings on the site vary from 65.38 to 65.53 m.

The upper quaternary alluvial deposits of the first floodplain terrace (a1III) and technogenic formations (t IV) are involved in the geological structure. In the section are highlighted (top to bottom):

Man-made soil (t IV):

Soil Unit-1 is a mechanical mixture of loam, sand, in the roof layer with small building debris.

Alluvial deposits (a2III):

Soil Unit-2 is sandy brown, hard sand, plastic interlayers;

Soil Unit-3 is brown, plastic sandy loam.

The groups of soils according to the difficulty of developing a single-bucket excavator according to GESN-81-02-01-2017 [12] (Appendix 1.1) are shown in Table 5.

Table 5. Difficulty groups of soils IGE Soil

	name	Category	Item	number
1	Technogenic soil	2	35v	
2	Sandy loam, solid	1	36b	
3	Soft sandy loam	1	36b	

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